The following responses from previous Steel Interchange columns have been received:

 ing - not a good situation. Both the top and bottom of the connection plate should be stiffened laterally and an effort made to laterally stabilize the beam end.

David T. Ricker, P.E.

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We have received several requests from architects/ engineers over the past several years asking us to determine the weldability of steel for older (circa 1900's) building rehabilitation projects. In such cases we recommend a chemical analysis (including a tramp element survey) of the material in question to determine, among other things, the carbon equivalent. This information is

The connection shown is a poor one. The bolt group at B can be made strong enough so that the plate can be considered an extension of the beam web, but in so doing we transmit any rotation in the beam end (due to beam deflection) to the column web at A. The weld at A will be subject to shear and moment. The column web, shown unstiffened, may relieve some of the moment by slight controlled buckling. The remaining moment cannot accurately be determined. The weld at A must take this moment, whatever it is, into account. Regardless, when a plate has to "reach out: to make a connection it is subject to twist-

New Questions

Listed below are questions that we would like the readers to answer or discuss.

If you have an answer or suggestion please send it to the Steel Interchange Editor, Modern Steel Construction, One East Wacker Dr., Suite 3100, Chicago, IL 60601-2001 or e-mail it to:

John W. McCann